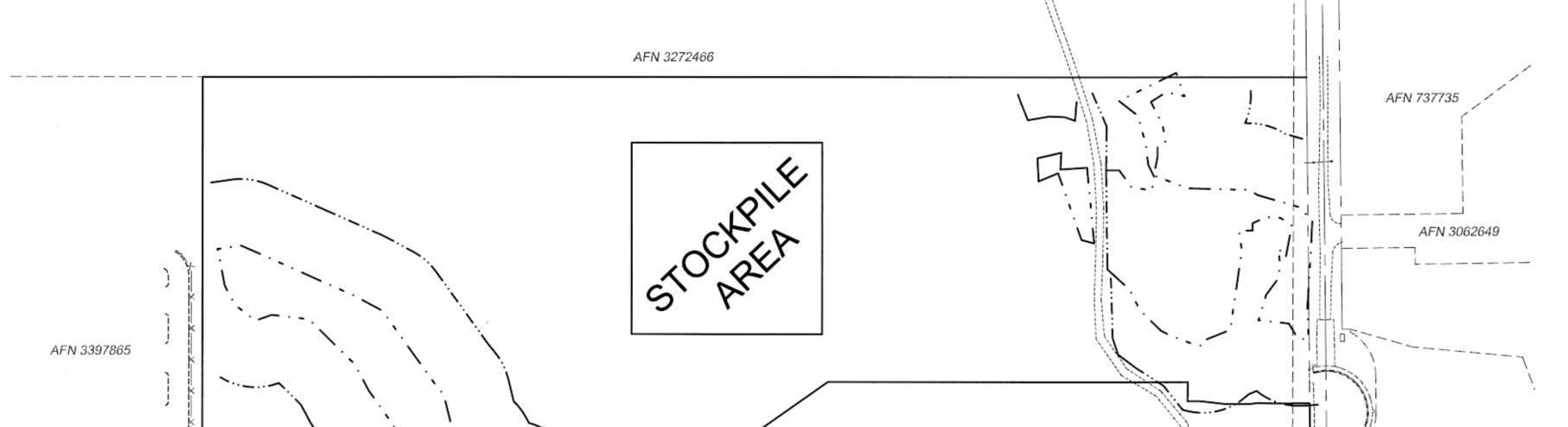
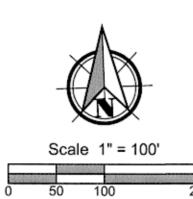
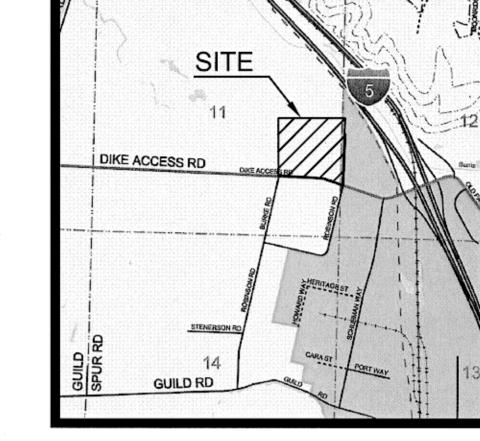
T. 5 N., R. 1 W., SEC 11 & 12



DIKE ACCESS ROAD

AFN 3279850





VICINITY MAP

Sheet Index					
C-001	COVER SHEET				
C-002	GENERAL NOTES				
C-003	MASTER LEGEND				
C-101	EXISTING CONDITIONS PLAN				
C-201	GRADING KEY & EROSION CONTROL PLAN				
C-202	STRIPPINGS AND STOCKPILE PLAN				
C-203	GRADING SURCHARGE PLAN				
C-204	GRADING SURCHARGE CROSS SECTION				
C-301 C-302	MISCELLANEOUS DETAILS STANDARD CITY OF WOODLAND DETAILS				

Project Summary:

LOT 1 SHORT SUBDIVISION NO. 206-911

LOT 2 SHORT

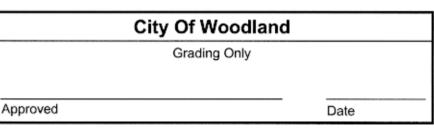
SUBDIVISION NO. 206-911

ROBIN

Phase 1 of the Woodland High School project involves the preparation of a building pad for the subsequent building project. Work in included in Phase 1 (this project) includes:

- a. Installation of erosion control measures, such as inlet protection, construction entrance with wheel wash, sediment basin, and silt fence;
- b. Clearing, mowing and striping of the site;
- c. Design and installation of a bidder designed Aggregate Pier Ground Improvement system; (the Aggregate Piers will be submited for permit separately from the grading permit.)
- d. Installation and protection of settlement monitoring
- e. Import, placement and compaction of structural fill; f. Import and placement of additional surcharge material;
- g. Continuous monitoring and maintenance of erosion
- control measures through the surcharge period.

The work of Phase 1 includes installation of surcharge material that is intended to compact the building pad in preparation for the subsequent building that will be constructed in Phase 2. The installation of the surcharge material will include installation of settlement monitoring plates which will allow the Owner's Geotechnical Engineer to monitor the progress of soil settlement through the surcharge period.



Owner: Woodland School District 112 800 3rd Street Woodland, WA 98674 Phone - (360) 841-2700

Architect: McGRANAHAN architects 2111 Pacific, Suite 100 Tacoma, WA 98402 Phone - (253) 383-3084

Civil Engineer / Surveyor: HDJ Design Group, PLLC. 300 W 15th Street, Suite 301 Vancouver, WA 98660

Geotechnical Engineer: Columbia West Engineering Inc. 11917 NE 95th Street, Vancouver, WA 98682 Phone - (360) 823-2900

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WOODLAND

COVER

C-001

All construction, workmanship and materials shall conform to the City of Woodland Construction Standards and the Washington State "Standard Specifications for Road, Bridge, and Municipal Construction 2012" unless revised here within.

All construction shall be in accordance with NPDES construction stormwater permit and the Stormwater Pollution Prevention Plan (SWPPP). The Contractor shall have a copy of the SWPPP on the construction site at all times. SWPPP logs shall be kept current and all required entries to date.

It shall be the responsibility of the Contractor to verify all utility locations prior to construction and arrange for the relocation of any in conflict with the proposed construction. The locations, depth and description of existing utilities shown on the Project Plans were compiled from available records and/or field surveys. The Owner or utility companies do not guarantee the accuracy or the completeness of such records. Additional existing utilities may exist within the work area.

The contractor shall post emergency telephone numbers for police, fire, ambulance, and those agencies responsible for the maintenance of utilities in the jobsite.

Should it appear that the work to be done or any matter relative there to is not sufficiently detailed or explained on the Phase 1 Grading Plans, the Contractor shall submit an RFI (Request for Information) to the Architect.

A geotechnical report has been prepared for the Woodland High School project by Columbia West Engineering (Project Geotechnical Engineer), dated November 20, 2012, titled "Geotechnical Site Investigation Woodland High School". It is the Contractor's responsibility to obtain a copy, review and familiarize him/herself with the report. For report interpretation and questions the Contractor shall submit an RFI (Request for Information) to the Architect.

Wetlands and wetland buffers are shown on the Phase 1 Grading Plans and shall be field marked by the Project Surveyor prior to construction of this project. Except where shown on the plans and marked in the field, the Contractor shall not disturb, place or allow any construction activities within the wetlands and wetland buffers.

Inadvertent Discovery

In the event any archaeological or historic materials are encountered during project activity, work in the immediate area (initially allowing for a 100' buffer; this number may vary by circumstance) must stop and the following actions

- 1. Implement Reasonable Measures to protect the discovery site, including any appropriate stabilizations or covering; and
- 2. Take reasonable steps to insure the confidentiality of the discovery site; and,
- 3. Take reasonable steps to restrict access to the site of discovery.

The project proponent will notify the concerned Tribes and all appropriate county, state, and federal agencies, including the Department of Archaeology and Historic Preservation. The agencies and Tribe(s) will discuss possible measures to remove or avoid cultural material, and will reach and agreement with the project proponent regarding actions to be taken and disposition of

If human remains are uncovered, appropriate law enforcement agencies shall be notified first, and the above steps followed. If the remains are determined to be Native, consultation with the affected Tribes will take place in order to mitigate the final disposition of said remains.

See the Revised Code of Washington, Chapter 27.53, "Archaeological Sites and Resources," for applicable state laws and statutes. See also Washington State Executive Order 05-05, "Archaeological and Cultural Resources." Additional state and federal law(s) may also apply.

Erosion Control:

Erosion control measures shall be installed before construction activities can commence.

The erosion control plan and details contained within the Phase 1 Grading Plans are a minimum requirement.

In addition to maintaining the erosion control devices, the Contractor shall upon completion of construction activities, maintain, repair to proper working order and/or replace erosion control, drainage, slope protection, and general site stability devices.

Grading:

Site Preparation

Before any work can commence, Contractor shall flag clearing limits of Structural Fill and Stockpile Areas.

Project Surveyor shall field mark existing wetlands and wetland buffers. Contractor to protect existing wetlands and wetland buffers at all times. All site preparation, topsoil stripping, and grading activities shall be observed and documented by the Project Geotechnical Engineer or it's designated representative.

All topsoil strippings are to remain on-site. Topsoil strippings shall be placed in the Stockpile Area in a neat manner and covered with plastic per the plan detail.

Prior to topsoil stripping activity, Structural Fill and Stockpile Areas shall be mowed leaving grass stubble not exceeding 2 inches in height. Non-grass vegetation, woody debris and deleterious materials shall be cleared from the Structural Fill and Stockpile Areas. Non-grass vegetation, woody debris and deleterious materials shall be removed, hauled and disposed of at a Contractor provided waste site.

The Structural Fill Area shall have topsoil stripped and topsoil shall be place in the Stockpile Area as noted on the plans. The Stockpile Area does not require stripping of topsoil and shall remain at existing grade. A topsoil stripping depth of approximately 6 inches is anticipated for much of the project site. The required topsoil stripping depth may increase if areas of highly organic soils are encountered. The actual depth of topsoil stripping shall be determined in the field by the Project Geotechnical Engineer during construction when subgrade conditions are exposed.

Unsuitable soil encountered during grading or construction activities shall be removed completely and thoroughly. Contractor shall contact the Project Geotechnical Engineer prior to removal. The Project Geotechnical Engineer shall determine if the above mentioned materials are suitable to be placed in the Stockpile Area or shall be removed from the project site and disposed of in the Contractor provided waste site.

During extended wet periods, topsoil stripping activities may need to be conducted from an advancing pad of granular fill material. Advancing pad material shall consist of granular import meeting WSDOT 9-03.14(1) or WSDOT 9-03.13(1) or other material approved by the Project Geotechnical Engineer. Advancing pad material shall be compacted in accordance with the Structural Fill compaction requirements.

The Contractor shall provide positive drainage to prevent surface ponding of water in the subgrade of the Structural Fill Area.

The Contractor shall repair softened or otherwise damaged subgrade soils that result from topsoil stripping, Aggregate Pier installation, or grading activities at no cost to the Owner.

Aggregate Pier Installation:

Aggregate pier installation shall be per the project specifications.

Working Aggregate Pad:

If the Contractor deems necessary the Contractor shall provide a Working Aggregate Pad capable of allowing Aggregate Pier installation equipment to operate and move safely and without excessive disturbance of subgrade soils.

Materials for a Working Aggregate Pad shall consist of import granular structural fill meeting WSDOT 9-03.14(1) or WSDOT 9-03.13(1) or other material approved by the Project Geotechnical Engineer, and shall be compacted to at least 95 percent of the modified Proctor dry density as determined by ASTM D1557.

Crushed surfacing base course meeting WSDOT 9-03.9(3) may be utilized as a wearing surface to cap the Working Aggregate Pad and shall be compacted to at least 95 percent of the modified Proctor dry density, as determined by ASTM D1557.

If stabilization below the Working Aggregate Pad is needed due to soft subgrade conditions, all-weather gravel, 2x4-inch gabion, or other similar material (six-inch maximum size with less than five percent passing the No. 200 sieve) shall be used.

Construction during wet weather may require increased Working Aggregate Pad thickness. Over-excavation may be necessary to provide a firm base upon which to place structural fill material. Geo-textile filter fabric may also be required and as approved by the Project Geotechnical Engineer.

Upon completion of Aggregate Pier installation, the Working Aggregate Pad shall be proof-rolled by the contractor and density tested by the Project Geotechnical Engineer according to the specifications indicated herein. Areas of the Working Aggregate Pad that do not meet testing requirements for structural fill shall be repaired at no cost to the Owner by the Contractor prior to acceptance by the Owner. In addition to the structural fill material, repair may include replacement of geosynthetic products, crushed aggregate, and over-excavation and backfill of disturbed or softened subgrade soil areas.

Settlement Plates:

Settlement plates shall be installed by the Contractor in accordance with the following specifications and Phase 1 Grading Plan details prior to placement of Structural Fill, Surcharge Fill or the installation of Aggregate Piers.

The specific location of the settlement plates shall be determined by the Project Geotechnical Engineer in the field. General locations are indicated on the Phase 1 Grading Plans. If a conflict arises with the layout of proposed Aggregate Piers, the Project Geotechnical Engineer shall be contacted to determine alternate locations for settlement plates.

Settlement mitigation shall be evaluated by periodically monitoring the elevation of settlement plates prior to fill material placement, during fill material placement, and after fill material placement. The schedule for documenting the elevation of the settlement plates by the Project Surveyor shall be as follows:

- Initial placement One time per week throughout the duration of installation of the Aggregate Piers.
- Two times per week during the placement of the Structural and Surcharge Fill extending 30 days past the completion of the of the Surcharge Fill.
- completion of the Surcharge Fill.

- One time per week for the period from 31 days to 60 days after the

- One time every other week for the period of 61 days to 90 days after the completion of the Surcharge Fill
- Each occurrence of a riser rod extension.

The Project Geotechnical Engineer may alter the survey schedule based upon measured displacement and actual rates of consolidation.

The following are specifications for settlement plate installation:

Care should be taken during construction not to disturb, alter, move, or damage the settlement plates.

Settlement plates shall be placed level upon neatly excavated subgrade soils. The riser rod should be maintained plumb throughout construction. Installation of settlement plates should be directed, overseen, and documented by the Project Geotechnical Engineer.

Settlement plates should consist of square steel plates with a vertical riser rods, dimensions as shown on the Settlement Plate detail and extend through the engineered Structural and Surcharge Fills.

The extension of the riser rods shall be conducted under the direct supervision of the Project Geotechnical Engineer. Survey of the existing top-of-riser and new top-of-riser shall be required at the time of extension to monitor any settlement that has already taken place.

Structural Fill:

Upon completion of Aggregate Pier installation and acceptance of the subgrade or Working Aggregate Pad as structural fill, the Contractor shall then place structural fill and geo-grid to the lines and grades designated on the Phase 1 Grading Plans.

Structural fill material shall consist of granular import meeting WSDOT 9-03.14(1) or WSDOT 9-03.13(1) or other material approved by the Project Geotechnical Engineer. Two weeks prior to placement of the structural fill, the Contractor shall provide representative samples of the proposed structural fill material to the Project Geotechnical Engineer for laboratory analysis and approval. Quantity of the structural fill samples to be determined by the Project Geotechnical Engineer.

Specified geo-grid products shall be placed as a part of the structural fill at the extents and elevations indicated on the Phase 1 Grading Plans. Geo-grid products shall consist of AllianceGeo BX2020 or approved equal meeting general requirements in WSDOT 9-33.

Structural fill shall be constructed by placing fill material in maximum 12-inch level lifts, compacting, and horizontally benching where appropriate. Structural fill material shall be compacted to at least 95 percent of the Modified Proctor dry density test, as determined by ASTM Compaction testing of structural fill shall be performed and verified by the Project Geotechnical Engineer using nuclear gauge field compaction testing performed in accordance with ASTM D6938.

In addition to nuclear gauge density testing, a wheel proof-roll shall be conducted by the Contractor using a 12-cubic yard, double-axle dump truck or equivalent. The Project Geotechnical Engineer shall be present to observe, verify and document the wheel proof rolling.

Field compaction testing and/or a proof-roll shall be performed for each vertical foot of structural fill placed. Structural fill placement shall be observed by the Project Geotechnical Engineer.

Fill slopes greater than six feet in height shall be vertically keyed into existing subsurface soil.

A minimum horizontal setback for long-term loads of 10 feet from top of cut or fill slope face shall be maintained.

Concentrated drainage or water flow over the face of slopes shall be prohibited, and adequate protection against erosion is required.

Surcharge Fill:

Upon completion of the Structural Fill placement by the Contractor and acceptance by the Project Geotechnical Engineer, the Contractor shall place the Surcharge Fill to the lines and grades designated on the Phase 1 Grading Plans. Placement of surcharge fill material shall be observed and documented by the Project Geotechnical Engineer. Surcharge fill material shall consist of granular import meeting WSDOT 9-03.14(1) or WSDOT 9-03.13(1) or other material approved by the Project Geotechnical Engineer. Two weeks prior to placement of the surcharge fill, the Contractor shall provide representative samples of the proposed surcharge fill material to the Project Geotechnical Engineer for laboratory analysis and approval. Quantity of the structural fill samples to be determined by the Project Geotechnical Engineer.

Surcharge fill shall be constructed by placing fill material in maximum 12-inch level lifts, track compacted, and horizontally benching where appropriate.

Side-slopes for temporary surcharge stockpiles shall not exceed a gradient of 1H:1V.

Concentrated drainage or water flow over the face of slopes shall be prohibited, and adequate protection against erosion is required.

Fill within three feet horizontally from the riser of a settlement plate shall be compacted to specifications with hand equipment.

Nominal 1-inch diameter threaded steel pipe should comprise riser rods. Riser rods should be protected from Structural and Surcharge Fill by encasement with PVC pipe per the detail.

Oakum or other approved product shall be used to fill the annulus space between the riser and the protective PVC within 12 inches of the settlement plate per the detail.

Riser rods should be maintained at least two feet above the fill level.

BENCHMARK:

HDJ DESIGN GROUP MAG NAIL #80 LOCATION: SET IN SIDEWALK 3.8 FEET NORTHEAST FROM THE FACE OF CURB OF THE NORTH ACCESS RAMP DROP AT THE NORTHEAST INTERSECTION OF DIKE ACCESS ROAD AND ELDON ROBINSON ROAD. ELEVATION: 16.31' DATUM: NAVD 88

THE ORIGINAL BENCHMARK USED WAS COWLITZ COUNTY BENCHMARK NUMBER 286 "CC GPS 1236" (1-1/4" IRON PIPE IN A MONUMENT BOX) AT THE INTERSECTION OF THE CENTERLINE OF DIKE ACCESS ROAD AND THE CENTERLINE OF DIKE ACCESS ROAD SOUTHBOUND OFF RAMP (EXIT 22) OF INTERSTATE 5.

*THE ORIGINAL BENCHMARK HAS SINCE BEEN DESTROYED DUE TO ROAD IMPROVEMENTS.

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NOTES GENERAL

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SHEET 2 OF 10

	ymbol	Legend	
Existing Water Valve	w> C×3	Proposed Catch Basins	
Existing Gas Valve	GV ⊠	Proposed Area Drain	0
Existing Fire Hydrant	- \$-	Proposed Combination Curb Inlet	
	-0-	Proposed Storm Reducer	•
Existing Power Pole		Proposed Rain Drain Proposed Storm Cleanout	
Existing Water Meter	83	Proposed Storm Manhole	ė
Existing Electrical Pedestal	Б	Proposed Sedimentation Manhole	0
Existing Power Riser		Proposed Drywell	•
Existing Power Meter	P _M	Proposed Sanitary Cap]
		Proposed Sanitary Reducer	>
Existing Sanitary Manhole	0	Proposed Sanitary Cleanout Proposed Sanitary Manhole	0
Existing Storm Manhole	(3)	Proposed Irrigation Meter	<u> </u>
Existing Catch Basin		Proposed Irrigation Backflow Device	
Existing Area Drain	€	Proposed Irrigation Valve	1891
Existing Combo Inlet	음	Proposed Irrigation Bend Tee W/valve	₩ <u>₹</u>
		Proposed Irrigation Bend Tee W/tb	
Existing Telephone Pad	P	Proposed Water 22½° Bend W/tb	<u>.</u>
Existing Telephone Riser	⋈	Proposed Water 11¼° Bend W/tb Proposed Irrigation 45° Bend W/tb	. ₹
Existing Roof Drain	Ψ	Proposed Irrigation 90° Bend W/tb	
Existing Cleanout	e CO	Proposed Irrigation Stand Pipe	
		Proposed Irrigation Bend X	®∰®
Existing Guy Anchor	(-	Proposed Irrigation Temporary Blowoff	D08
Existing Project Bench Mark	*	Proposed Irrigation Standard Blowoff	№.
Existing Iron Rod	0	Proposed Irrigation Reducer	Þ
Existing Sign	Ч	Proposed Irrigation Thrust Block	₽
Existing Shrub	99	Proposed Inlet Protection Pillow	
Existing Deciduous Tree	0		_
Existing Coniferous Tree	於	Proposed Gravel Construction Entrance	<i>6</i> 88
See Extg. Sanitary Sewer Data	\otimes		<u>6</u>
See Extg. Storm Drainage Data	X	Proposed Sedimentation Trap	
Existing Flow Arrow	$igcap {igl[}$	Erosion Control feature code	
Proposed Bollard	۰	& ID number (Puget Sound)	E 3.3
Proposed Street Light	← ×	BMP Type (Puget Sound)	CIP-1
Proposed Road Barrier	<u> </u>	Future Storm Manhole	0
Proposed Road Sign Proposed Flow Arrow	<u>-</u>	Future Sanitary Manhole	0
Proposed Fire Protection Vault	-	Future Fire Hydrant	304
Proposed Water Meter	<u> </u>	Future Catch Basin	
Proposed Water Backflow Device		Future Sanitary Cap	Į.
Proposed Water Valve	181	Future Fire Protection Vault	8
Proposed Water Bend Tee W/valve	ø <u>⊺</u> ∢	Future Water Meter	10000
Proposed Water Bend Tee W/tb		Future Backflow Device Future Valve	100
Proposed Water 22½° Bend W/tb	<u>.</u> ∢	Future Bend Tee W/valve	101 -63
Proposed Water 11¼° Bend W/tb Proposed Water 45° Bend W/tb	্ধ	Future Bend Tee W/tb	
Proposed Water 90° Bend W/tb		Future 22½° Bend W/tb	4
Proposed Water Stand Pipe	×4	Future 111/4° Bend W/tb	4
Proposed Water Bend X	S S B	Future 45° Bend W/tb	্ৰ
Proposed Water Temporary Blowoff	Dos.	Future 90° Bend W/tb	⟨ৠ
Proposed Water Standard Blowoff	8	Future Stand Pipe	×
Proposed Water Reducer	•	Future Bend X	<u>⊚</u> 10
Proposed Water Thrust Block	₽	Future Temporary Blowoff	(B) (S)
Proposed Fire Hydrant	₩	Future Standard Blowoff	≥8
		Future Reducer Future Thrust Block	A A
		I ULUI O I III USL DIUUK	122

Future Fire Hydrant

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Without

Yard

Water Meter

With Yellow Plastic Cap

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Hatching Legend						
Proposed Asphalt Concrete Proposed Cement Concrete Proposed Gravel Road						

				Abb	reviati	on Legend			
Acres	AC	Compaction	COMP	Finished Floor	FF	Maximum	MAX	Stainless Steel	SS
Assembly	ASS'Y	Concrete	CONC	Finished Grade	FG	Manhole	МН	Steel	STL
Avenue	AVE	Construction	CONST	Fire Hydrant	FH	Minimum	MIN	Sidewalk	S/W
Approved	APP'D	Corrugated Polyethylene	CPE	Flange	FLG	Mechanical Joint	MJ	Street	ST
Butterfly	BF	Concrete Sewer Pipe	CSP	Force Main	FM	Number	No. or #	Station Centerline	STA
Boulevard	BLVD	Court	CT	Foot / Feet	FT	Overhead Electric	OHE	Standard	STD
Benchmark	ВМ	Cubic Yard	CY	Gas	G	Pavement	PAV'T	Sanitary	SAN
Blow Off	во	Cement	CEM	Galvanized Iron	GI	Point Of Curve	PC	Storm	STM
Back Of Curb	BOC	Depth	D	Ground	GRD	Power Pole	PP	Tangent	Т
Begin Vertical Curve	BVC	Ductile Iron	DI	Gate Valve	GV	Point Of Reverse Curve	PRC	Thrust Block	ТВ
Care Of	C/O	Diameter	DIA	High Density Polyethylene	HDPE	Point Of Reverse Vertical Curve	PRVC	Temporary Benchmark	TBM
Catch Basin	CB	Ductile Iron Pipe	DIP	Horizontal	HORIZ	Point Of Tangent	PT	Top Of Curb	TC
Cubic Feet	CF	Down Spout	DS	High Water Elevation	HW	Point Of Vertical Intersection	PVI	Telephone	TEL
Cast Iron	CI	Edge Of Pavement	EOP	Hydrant	HYD	Polyvinyl Chloride	PVC	Temporary	TEMP
Cement	CEM	End Curb Return	ER	Invert Elevation	IE	Place	PL	Top Of Manhole	TOP
Circle	CIR	Easement	ESMT	Intersection	INTX	Radius	R	Typical	TYP
Centerline	ę	Existing	EXTG	Invert	INV	Right Of Way	R/W	Underground Electric	UGE
Corrugated Metal Pipe	CMP	Elevation	EL	Length	L	Return	RET	Vertical Curve	VC
Cleanout	СО	Electric	ELEC	Lateral	LAT	Right	RT	Vertical	VERT
Combination	СОМВ	End Vertical Curb	EVC	Left	LT	Sheet	SHT	Water	WTR

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SHEET **3** OF **10**

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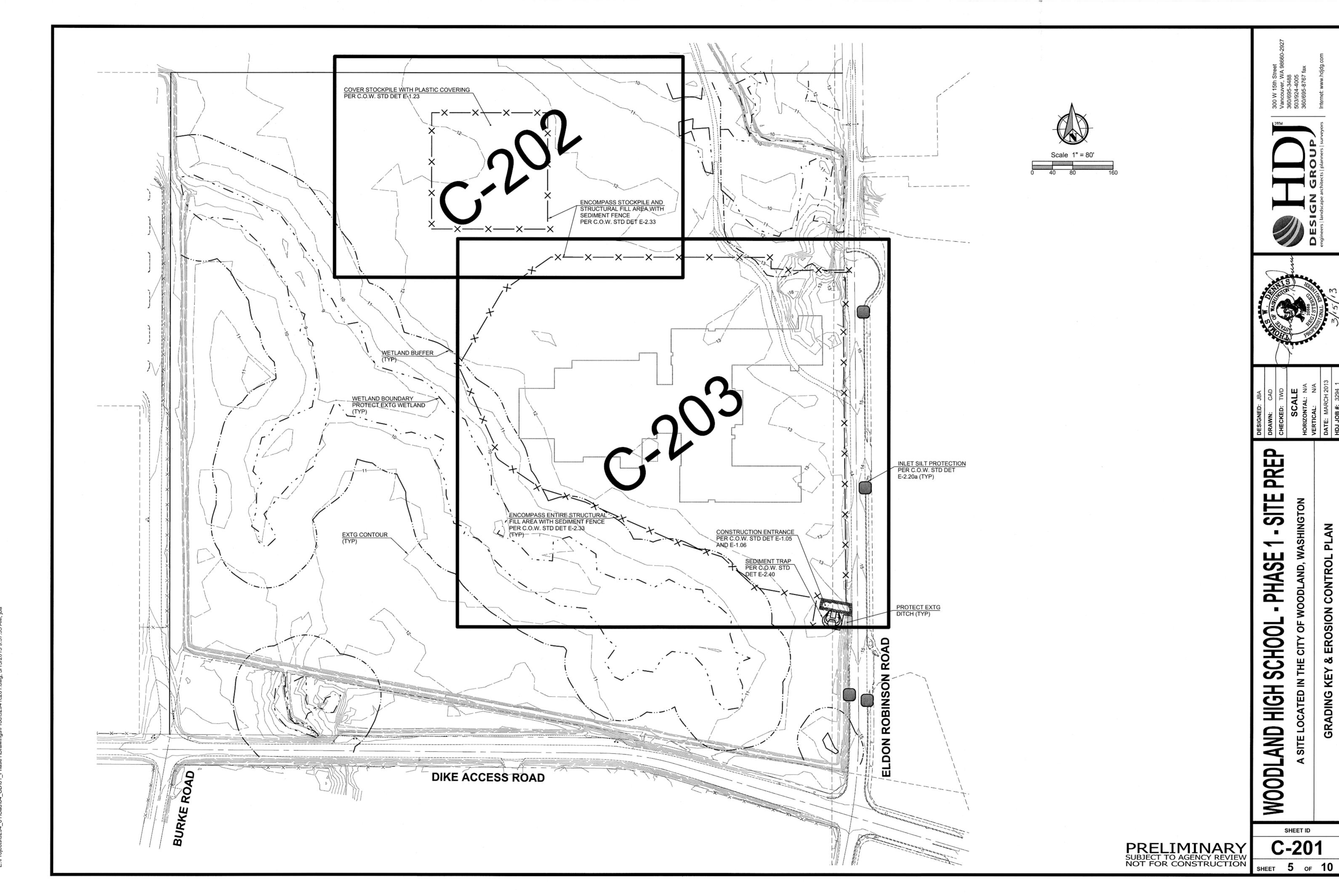
EXISTING CONDITIONS

A SITE LOCATED IN

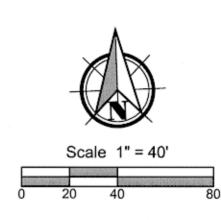
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SHEET 4 OF 10



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C-202
SHEET ID

C-202
SHEET 6 OF 10

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PHASE

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WOODLAND HIGH

WOODLAND, W

SITE LOCATED IN THE CITY OF

STRIPPINGS AND STOCKPILE PLAN

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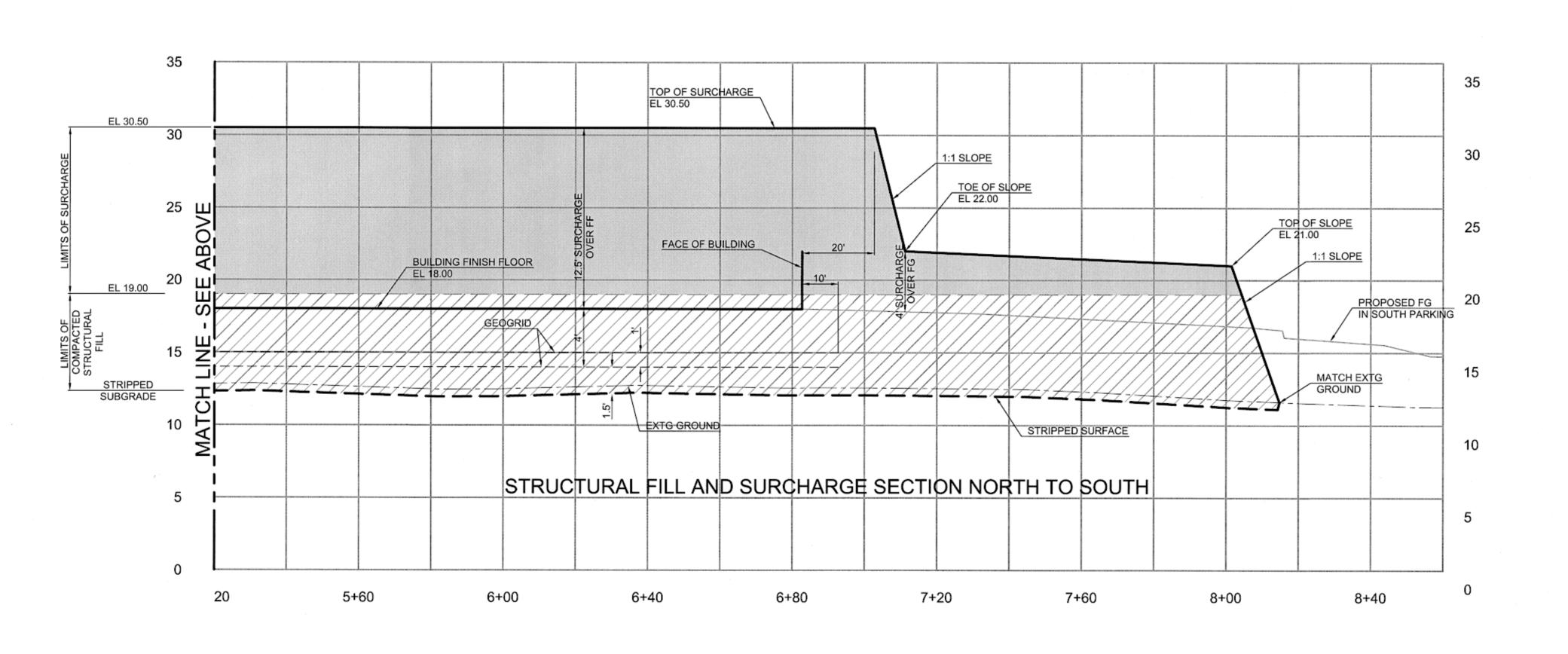
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SURCHARGE PLAN

SHEET ID

C-203

SHEET **7** OF **10**



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CROSS SECTION

GRADING SURCHARGE

SHEET ID C-204

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PHASE

WOODLAND HIGH SCHOOL

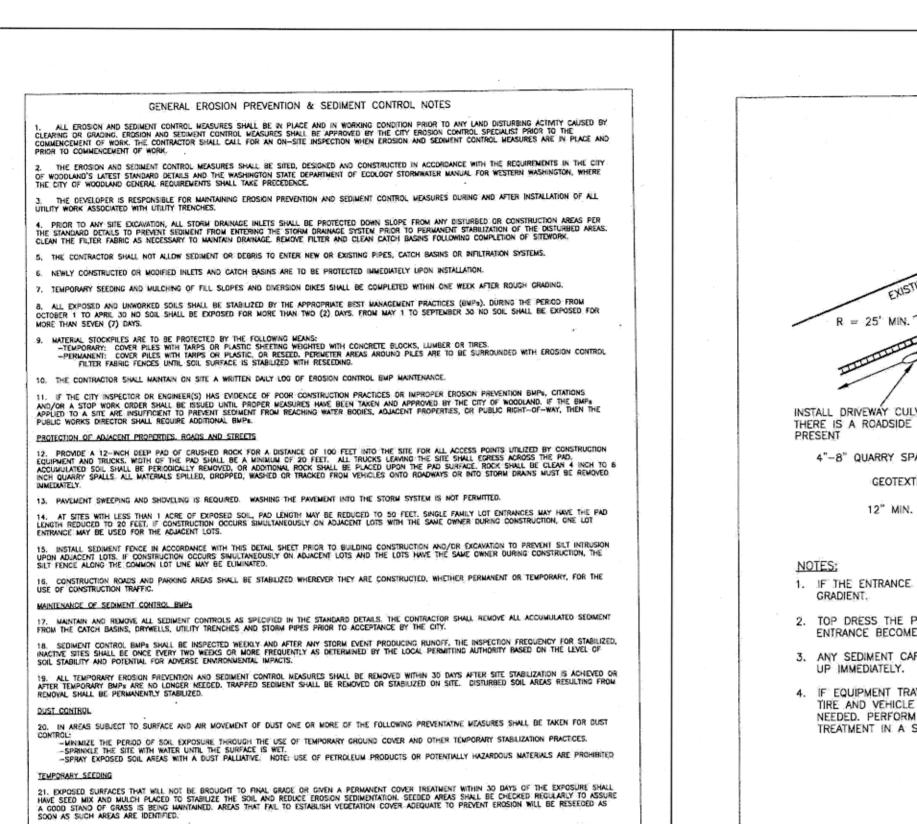
WOODLAND, WASHINGTON

MISCELLANEOUS DETAILS

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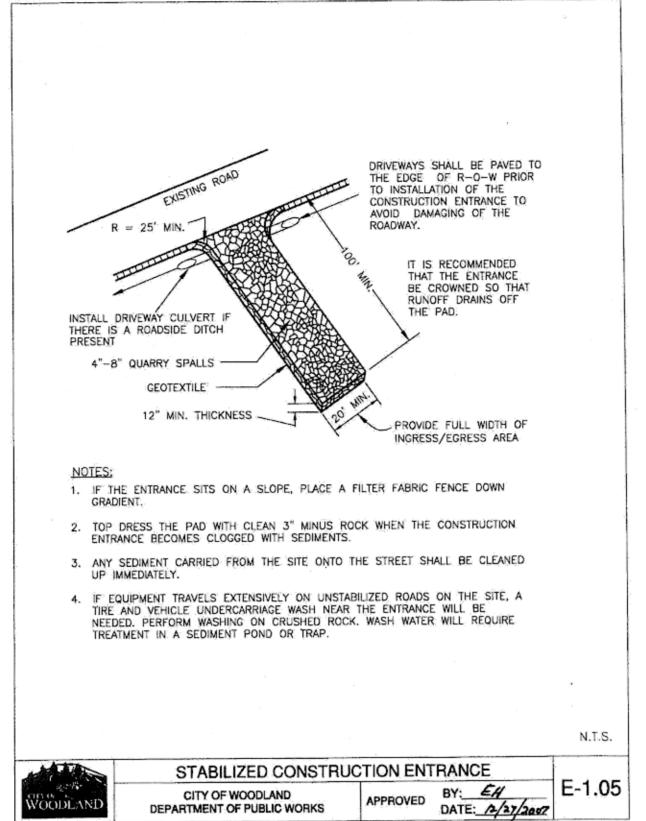
C-301

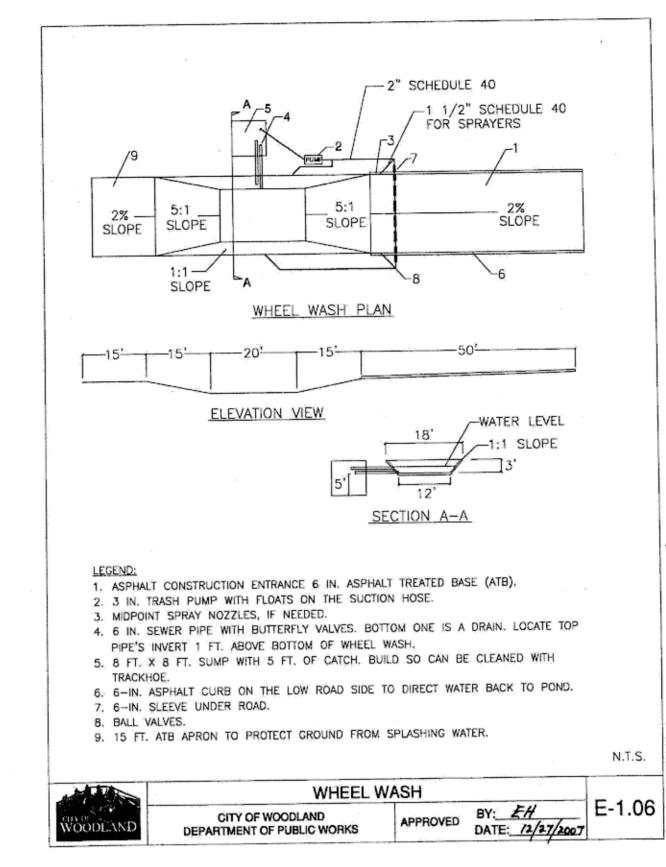
SHEET 9 OF 10

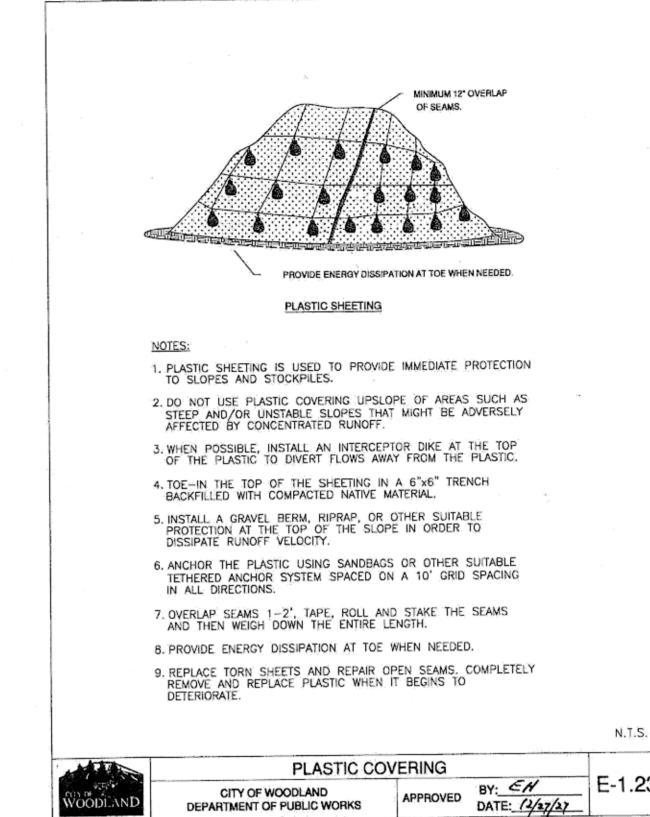


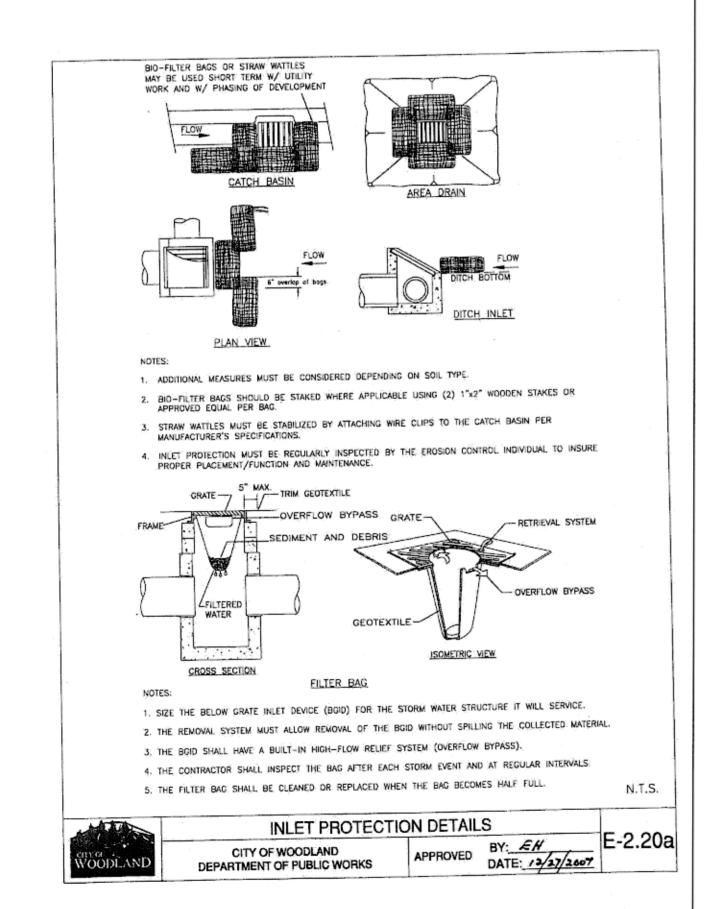
BY: EH

DATE: 14/17/1607





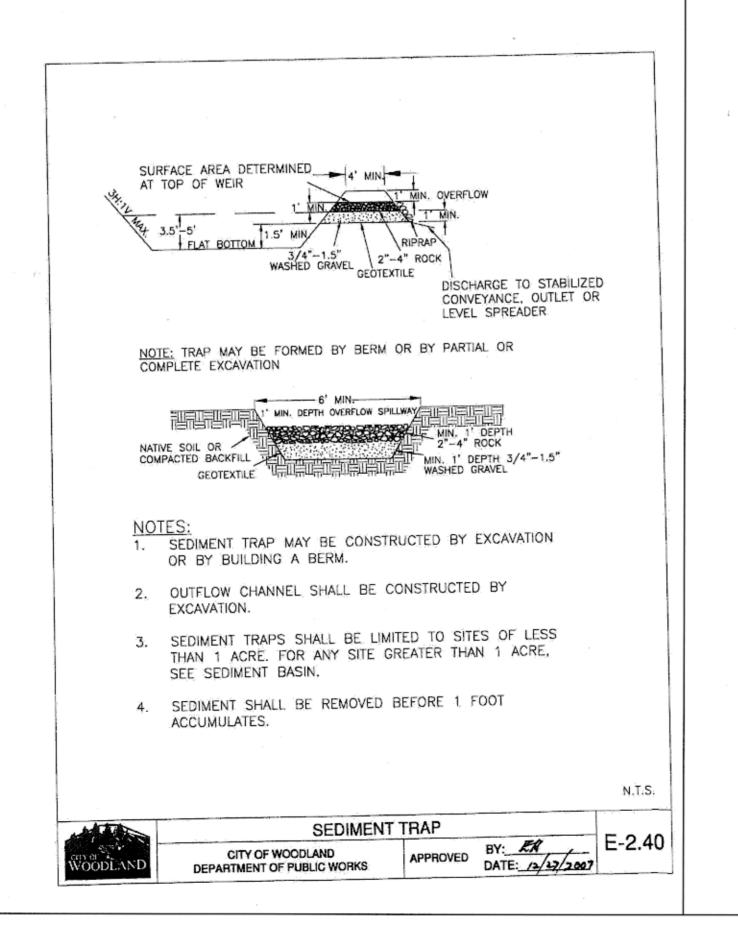


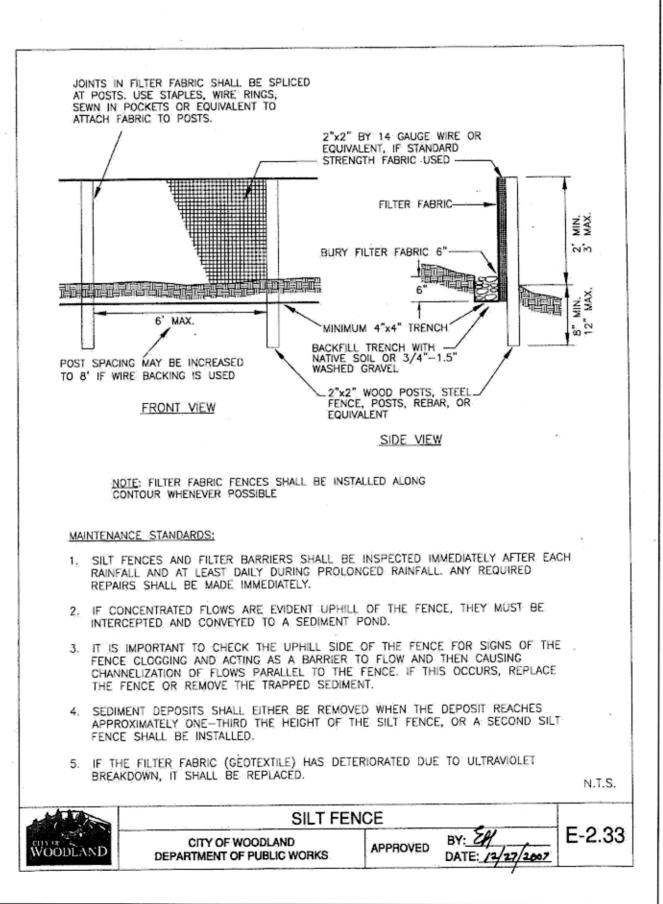


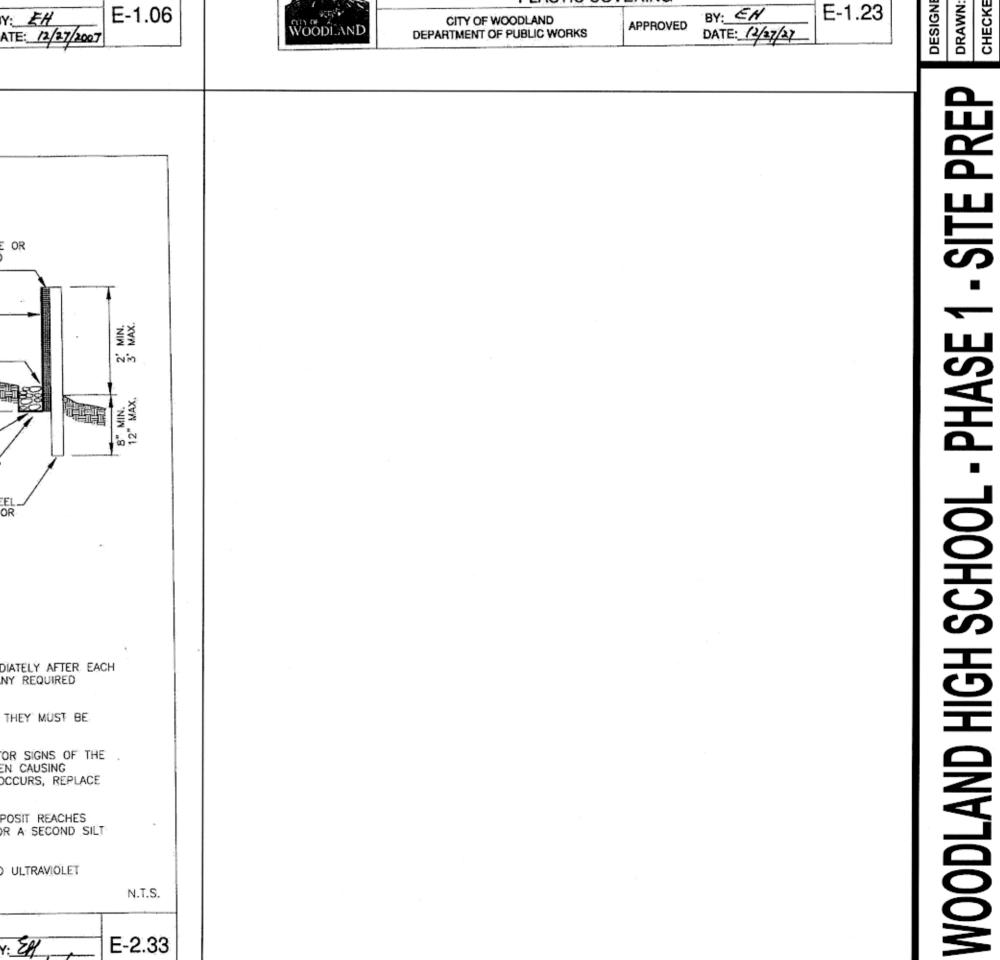
22. APPLY AN APPROVED TEMPORARY SEEDING MIXTURE TO THE PREPARED SEED BED AT A RATE OF 120 LBS/ACRE. NOTE: "HYDROSEEDING" APPLICATIONS WITH APPROVED SEED-MULCH-FERTILIZER MIXTURES MAY ALSO BE USED.

DEPARTMENT OF PUBLIC WORKS

EROSION PREVENTION & SEDIMENT CONTROL







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SHEET 10 OF 10